

FINITE ELEMENT METHOD

ФИНИТ ЕЛЕМЕНТ МЕТОД

18

Differential
Equation *

FINITE ELEMENT METHOD

FINITE ELEMENT METHOD

STRONG FORM

Strong to Weak Form

WEAK FORM

Weak to Approximate Form

APPROXIMATE FORM

From Physical to Natural Space

NUMERICAL EVALUATION (Integration)

Approximate Solution to Differential Equation *

ROADMAP

FOR FEM

1D
2D

DISCRETIZED FORM

APPROXIMATION TECHNIQUES
↳ SHAPE FUNCTIONS

UNDERSTANDING FEM VIA AN ANALOGY (A BRUTAL SIMPLIFICATION)

Approximations
in
FEM

- Solution Approximation → inherent to numerical techniques
- Equation Approximation → diff equation using computers
- Input Approximation → space transformed
discretization to weak form + space approximation



DOMAIN (X)
Discretization (Approximation)
(Solution (u))
REST (w)

diff. Eq. ↓
STRONG FORM
integral TO
WEAK FORM

FINITE ELEMENT METHOD IN A NUTSHELL

Diff. Eq. \rightarrow 2^{ND.} O.D.E.

STRONG FORM

$$\int_0^L (EAu')' + b = 0$$

another source of approximation \rightarrow NUMERICAL INTEGRATION

ELEMENT-WISE QUANTITIES

PIECEWISE INTEGRALS (Solutions)

\rightarrow (I) Multiply By w \rightarrow (II) INTEGRATE

test function

Approximate Discretized Weak Form

APPROXIMATE FORM

WEAK FORM

INTEGRAL FORM

$$\int_0^1 w'u' dx = \int_0^1 w da$$

$$+ w(1)u'(1)$$

$$- w(0)u'(0)$$

PIECEWISE

DISCRETIZED FORM

Approximation

PostProcess

SOLVE

From Global To Elements

From INTEGRAL OVER THE DOMAIN

ASSEMBLY

$$\int_0^1 \dots dx = \int_a^b \dots dx + \dots$$

$$[K][w] = [F]$$

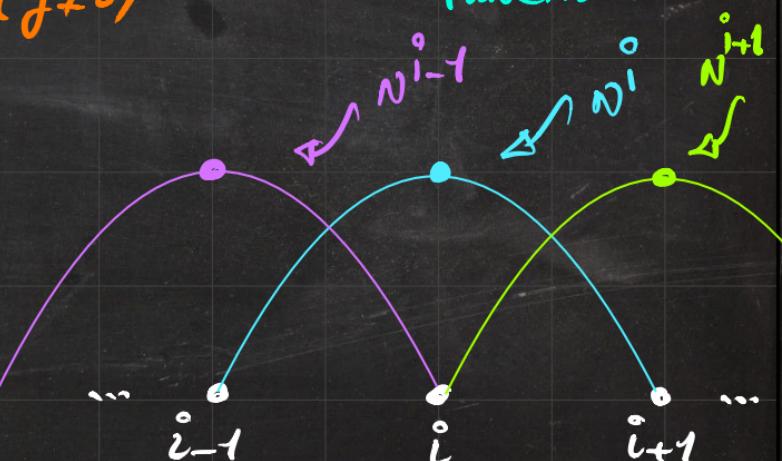
SHAPE FUNCTIONS (HAT Functions , TENT Functions)

↳ A powerful tool for approximations → SYSTEMATIC

$$N^i(x) \rightarrow \begin{cases} N^i = 1 @ x^j (j=i) \rightarrow \text{NEARLY IDENTICAL FOR 2D & 3D} \\ N^i = 0 @ x^j (j \neq i) \end{cases}$$



QUADRATIC HAT FUNCTIONS



SHAPE FUNCTIONS (HAT FUNCTIONS, TENT FUNCTIONS)

↳ A powerful tool for approximations \rightarrow SYSTEMATIC

$$N^i(x) \rightarrow \begin{cases} N^i = 1 @ x^j (j=i) & \rightarrow \text{NEARLY IDENTICAL FOR 2D} \\ N^i = 0 @ x^j (j \neq i) & \text{3D} \end{cases}$$

linear
approximation

NODES PER ELEMENT \rightarrow NPE

$$u \cong \sum_{i=1}^n N^i u^i \rightarrow \begin{cases} u = N^1 u^1 + N^2 u^2 & \checkmark \text{ quadratic approximation} \\ u = N^1 u^1 + N^2 u^2 + N^3 u^3 & \checkmark \text{ approximation} \\ u = N^1 u^1 + N^2 u^2 + N^3 u^3 + N^4 u^4 & \checkmark \text{ cubic approximation} \end{cases}$$

GENERAL STRUCTURE OF STIFFNESS MATRIX FOR 1D FINITE ELEMENTS

$$K = EA \begin{bmatrix} NPE \times NPE \end{bmatrix}$$

NPE: Node Per Element
↓
 $[NPE \times PD] \times [NPE \times PD]$

1D	2D
LINEAR	2×2
TRUSS	4×4
QUADR. TRUSS	3×3 6×6

$$K = EA \begin{bmatrix} \int_{\alpha}^{\beta} N_1' N_1' d\alpha & \int_{\alpha}^{\beta} N_1' N_2' d\alpha \\ \int_{\alpha}^{\beta} N_2' N_1' d\alpha & \int_{\alpha}^{\beta} N_2' N_2' d\alpha \end{bmatrix}$$

↖ LINEAR TRUSS ELEMENT

$$K^{ij} = EA \int_{\alpha}^{\beta} N_i^{j'} N_j^{i'} d\alpha$$

GENERAL STRUCTURE OF STIFFNESS MATRIX FOR 1D FINITE ELEMENTS

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 $[NPE \times PD] \times [NPE \times PD]$

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QUADRATIC TRUSS ELEMENT

$K^{ij} = EA \int_{\alpha}^{\beta} N^i' N^j' dx$

GENERAL STRUCTURE OF STIFFNESS MATRIX FOR 1D FINITE ELEMENTS

$$[K] = EA \begin{bmatrix} NPE \times NPE \end{bmatrix}$$

NPE: Node Per Element
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1D	2D
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QUADR. TRUSS	3×3 6×6

$$K^{ij} = EA \int_{\alpha}^{\beta} N^i N^j' dx$$

$N^i = N^i(\alpha)$ $N^j = N^j(\alpha)$
 $N^i' = N^i'(\alpha)$ $N^j' = N^j'(\alpha)$

EVALUATE THIS INTEGRAL

$$= EA \int_{\alpha}^{\beta} f(x) dx$$

numerically

NUMERICAL INTEGRATION :

$$\int_{-1}^1 g(\xi) d\xi = 2 \times g(0)$$

✓ GAUSS QUADRATURE FORMULA

↳ GAUSS POINTS

g : LINEAR

$$\int_{-1}^1 g(\xi) d\xi = g\left(\frac{1}{\sqrt{3}}\right) + g\left(-\frac{1}{\sqrt{3}}\right)$$

g : QUADRATIC

$$\int_{-1}^1 g(\xi) d\xi = g\left(\frac{1}{\sqrt{3}}\right) + g\left(-\frac{1}{\sqrt{3}}\right)$$

g : CUBIC

NUMERICAL INTEGRATION :

QUADRATURE RULE \rightarrow Gauss Points

$$\int_{-1}^1 g(\xi) d\xi = \sum_{i=1}^{GPE} \alpha_i g(\xi_i)$$

\uparrow weight factor

GPE \leq NUMBER OF GAUSS POINTS PER ELEMENT QUADRATURE POINTS
 $GPE > \frac{P+1}{2}$ ORDER OF POLYNOMIAL

	P	GPE	ξ_1 , α_1	ξ_2 , α_2	ξ_3 , α_3	GPE
LINEAR	1	1	$\xi_1 = 0$, $\alpha_1 = 2$			
QUADRATIC	2	2	$\xi_1 = -\sqrt{3}/2$, $\alpha_1 = 1$	$\xi_2 = \sqrt{3}/2$, $\alpha_2 = 1$		$\sum_{i=1}^{GPE} \alpha_i = 2$
CUBIC	3	2	$\xi_1 = -\sqrt{3}/2$, $\alpha_1 = 1$	$\xi_2 = \sqrt{3}/2$, $\alpha_2 = 1$		
4th. O.	4	3	$\xi_1 = -\sqrt{0.6}$, $\alpha_1 = 5/q$	$\xi_2 = 0$, $\alpha_2 = 8/q$	$\xi_3 = \sqrt{0.6}$, $\alpha_3 = 5/q$	
5th. O.	5	3				

NUMERICAL INTEGRATION :

$$\int_{-1}^1 g(\xi) d\xi = \sum_{i=1}^{GPE} \alpha_i g(\xi_i)$$

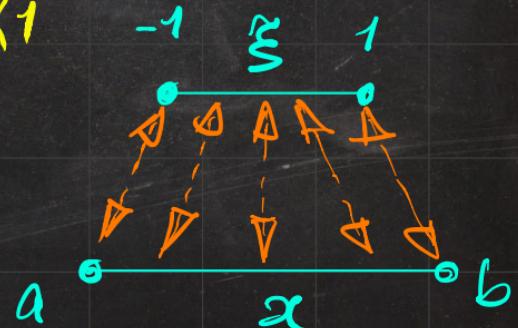
$$\int_a^b f(x) dx = ? \quad \swarrow \text{TRANSFORM TO} \quad \int_{-1}^1 g(\xi) d\xi \quad \uparrow$$

x -DOMAIN \swarrow PHYSICAL SPACE

$$a \leq x \leq b$$

ξ -DOMAIN \swarrow NATURAL SPACE

$$-1 \leq \xi \leq 1$$



MAPPING FROM PHYSICAL TO NATURAL SPACE

$$\hookrightarrow x \doteq x(\xi) \Rightarrow f(x) = f(x(\xi)) = g(\xi)$$

NUMERICAL INTEGRATION :

$$\int_{-1}^1 g(\xi) d\xi = \sum_{i=1}^{GPE} \alpha_i g(\xi_i)$$

$$\int_a^b f(x) dx = ? \quad \xrightarrow{\text{TRANSFORM TO}} \int_{-1}^1 g(\xi) d\xi \quad \begin{matrix} \nearrow J = \frac{\partial x}{\partial \xi} \\ \searrow \text{Jacobian} \end{matrix}$$

\curvearrowleft Polynomial ORDER

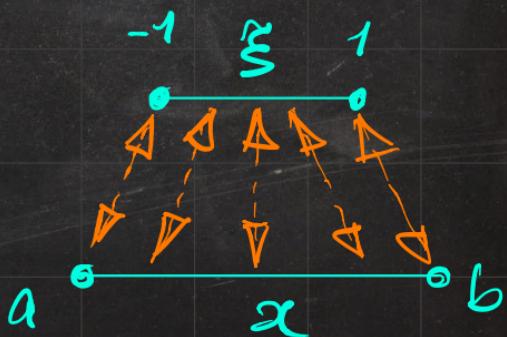
$$x \stackrel{?}{=} x(\xi) \Rightarrow x = \sum_{i=1}^{P+1} N^i \overset{\circ}{x}^i \quad \dots \quad dx = J(\xi) d\xi$$

$N^i = N^i(\xi)$

Shape Functions and $u = \sum N^i u^i$ values

\downarrow

$u(x)$ $\{N^i(x)\}$



NUMERICAL INTEGRATION :

$$\int_{-1}^1 g(\xi) d\xi = \sum_{i=1}^{GPE} \alpha_i g(\xi_i)$$

$\int_a^b f(x) dx = ?$ ↗ TRANSFORM TO $\int_{-1}^1 g(\xi) d\xi$

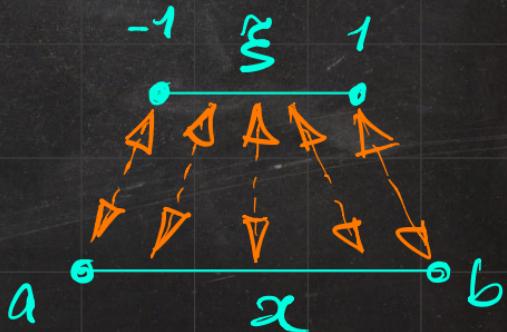
$J = \frac{\partial x}{\partial \xi}$

↗ Jacobian

$x \doteq x(\xi) \Rightarrow x = \sum_{i=1}^{P+1} N^i x^i$... $dx = J(\xi) d\xi$

↗ Polynomial ORDER
Nⁱ = Nⁱ(ξ)

$\int_a^b f(x) dx = \int_{-1}^1 f(x(\xi)) \underbrace{f(x(\xi))}_{g(\xi)} J(\xi) d\xi = \sum_{i=1}^{GPE} \dots$



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↳ SHAPE FUNCTIONS

1D FEM

Overviews and Wrap-up

1D FEM

Overviews and Wrap-up

WE Focus on LINEAR APPROXIMATIONS FOR THE SAKE OF CLARITY

↳ immediately relevant to classical truss elements

EINSTEIN SUMMATION CONVENTION

↗ A little definition for
notation convenience



• A REPEATED INDEX TWICE MEANS SUMMATION OVER THAT INDEX

EINSTEIN SUMMATION CONVENTION

↗ A little definition for
notation convenience



• A REPEATED INDEX TWICE MEANS SUMMATION OVER THAT INDEX

$$\sum_{i=1}^3 u_i v_i = u_1 v_1 + u_2 v_2 + u_3 v_3 \equiv u_i v_i$$

EINSTEIN SUMMATION CONVENTION

↗ A little definition for
notation convenience



• A REPEATED INDEX TWICE MEANS SUMMATION OVER THAT INDEX

↗ also, called "dummy index"

$$\sum_{i=1}^3 u_i v_i = u_1 v_1 + u_2 v_2 + u_3 v_3 \equiv u_i v_i \quad \text{↗ } i \text{ is summation index}$$

EINSTEIN SUMMATION CONVENTION

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A REPEATED INDEX TWICE MEANS SUMMATION OVER THAT INDEX

also, called "dummy index"

$$\sum_{i=1}^3 u_i v_i = u_1 v_1 + u_2 v_2 + u_3 v_3 \equiv u_i v_i \quad i \text{ is summation index}$$

$$\sum_{\substack{j=1 \\ 1 \leq i \leq 3}}^{i=3} A_{ij} u_j \Rightarrow \begin{cases} i=1 \Rightarrow A_{11} u_1 + A_{12} u_2 + A_{13} u_3 \\ i=2 \Rightarrow A_{21} u_1 + A_{22} u_2 + A_{23} u_3 \\ i=3 \Rightarrow A_{31} u_1 + A_{32} u_2 + A_{33} u_3 \end{cases} \Rightarrow \begin{bmatrix} A_{11} & A_{12} & A_{13} \\ A_{21} & A_{22} & A_{23} \\ A_{31} & A_{32} & A_{33} \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \\ u_3 \end{bmatrix} = A_{ij} u_j$$

EINSTEIN SUMMATION CONVENTION

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A REPEATED INDEX TWICE MEANS SUMMATION OVER THAT INDEX

also, called "dummy index"

$$\sum_{i=1}^3 u_i v_i = u_1 v_1 + u_2 v_2 + u_3 v_3 \equiv u_i v_i \quad i \text{ is summation index}$$

i : free index

$$\sum_{\substack{j=1 \\ 1 \leq i \leq 3}}^{i=3} A_{ij} u_j \Rightarrow \begin{cases} i=1 \Rightarrow A_{11} u_1 + A_{12} u_2 + A_{13} u_3 \\ i=2 \Rightarrow A_{21} u_1 + A_{22} u_2 + A_{23} u_3 \\ i=3 \Rightarrow A_{31} u_1 + A_{32} u_2 + A_{33} u_3 \end{cases} \Rightarrow \begin{bmatrix} A_{11} & A_{12} & A_{13} \\ A_{21} & A_{22} & A_{23} \\ A_{31} & A_{32} & A_{33} \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \\ u_3 \end{bmatrix} = A_{ij} u_j$$

j : summation index

SHAPE FUNCTIONS (LINEAR)

APPROXIMATE Solution (Disp.)

$$\{u\} = N^i \{u^i\}_{\text{node}} u(\alpha)$$

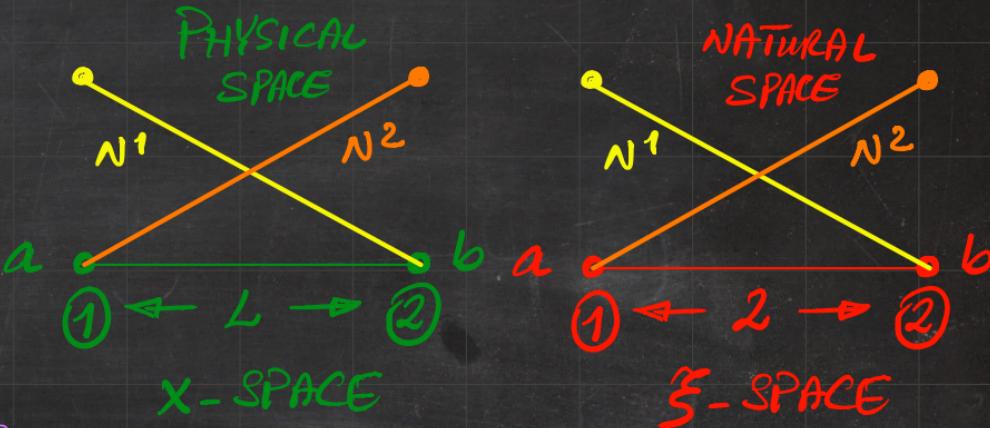
APPROXIMATE TEST Functions

$$\{\omega\} = N^j \{\omega^j\}_{\text{node}} \omega(\alpha)$$

APPROXIMATING THE SPECIE

$$\{x\} = N^k \{x^k\}_{\text{node}} x(\xi)$$

$$\Rightarrow \{\cdot\}_0 \{\cdot'\}_0 = \frac{\partial}{\partial x} \{\cdot\}_0$$



$$N^1 = -\frac{1}{L} [\alpha - b]$$

$$N^2 = \frac{1}{L} [\alpha - a]$$

$$N^1 = -\frac{1}{2} [\xi - 1]$$

$$N^2 = \frac{1}{2} [\xi + 1]$$

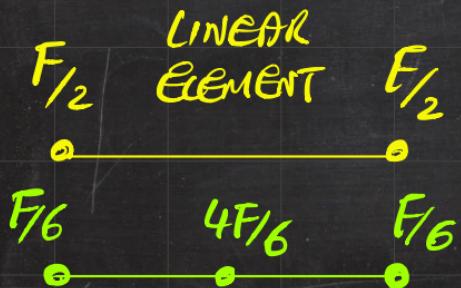
From STRONG FORM TO ELEMENT STIFFNESS \rightarrow IN PHYSICAL SPACE now

$$EAu'' + b = 0$$



\hookrightarrow NEGLECT THE BODY FORCE DENSITY

\hookrightarrow CAN BE TRANSFORMED TO EFFECTIVE



QUADRATIC ELEMENT

NODAL VALUES

\hookrightarrow e.g. a uniform body force density

$$F = b_o L \Leftarrow b_o$$



From STRONG FORM TO ELEMENT STIFFNESS IN PHYSICAL SPACE now

$$EAu'' = 0 \quad \text{SUBJECT TO BCs}$$



$\left. \begin{array}{l} \text{DIRICHLET} \rightarrow u \text{ is PRESCRIBED} \\ \text{NEUMANN} \rightarrow u' \text{ is PRESCRIBED} \end{array} \right\}$

From STRONG FORM TO ELEMENT STIFFNESS IN PHYSICAL SPACE now

$$EAu'' = 0 \quad \text{SUBJECT TO BCs}$$

 MULTIPLY BY
TEST
FUNCTION
 ω


$$EA\omega u'' = 0$$

 } DIRICHLET $\rightarrow u$ is PRESCRIBED
} NEUMANN $\rightarrow u'$ is PRESCRIBED



From STRONG FORM TO ELEMENT STIFFNESS IN PHYSICAL SPACE now

$$EA u'' = 0 \quad \text{SUBJECT TO BCs}$$


 MULTIPLY BY
 TEST
 FUNCTION
 ω


 } DIRICHLET $\rightarrow u$ is PRESCRIBED
 } NEUMANN $\rightarrow u'$ is PRESCRIBED



$$EA\omega u'' = 0 \quad \leftarrow \omega u'' = (\omega u')' - \omega u'$$

$$EA [(\omega u')' - \omega u'] = 0 \Rightarrow EA \omega' u' = EA (\omega u')'$$

From STRONG FORM TO ELEMENT STIFFNESS IN PHYSICAL SPACE now

$$EA u'' = 0 \quad \text{SUBJECT TO BCs}$$


 Multiply by
 TEST
 FUNCTION
 ω



 } DIRICHLET $\rightarrow u$ is PRESCRIBED
 } NEUMANN $\rightarrow u'$ is PRESCRIBED



$$EA\omega u'' = 0 \quad \leftarrow \omega u'' = (\omega u')' - \omega u'$$

$$EA [(\omega u')' - \omega u'] = 0 \Rightarrow EA \omega' u' = EA (\omega u')' \quad \leftarrow \text{INTEGRATE}$$

$$\int_L EA \omega' u' dx = \int_L EA (\omega u')' dx = EA \omega u' \Big|_1^2 = EA \omega u'^2 - EA \omega u'^1$$

From STRONG FORM TO ELEMENT STIFFNESS  IN PHYSICAL SPACE now

$$EA u'' = 0 \quad \text{SUBJECT TO BCs}$$



$$\int_L EA \omega' u' dx = \int_L EA (\omega u')' dx = EA \omega u' \Big|_1^2 = EA \omega^2 u'^2 - EA \omega^1 u'^1$$

From STRONG FORM TO ELEMENT STIFFNESS IN PHYSICAL SPACE now

$$EA u'' = 0 \quad \text{SUBJECT TO BCs}$$



$$\int_L EA \omega' u' dx = \int_L EA (\omega u')' dx = EA \omega u' \Big|_1^2 = EA \omega^2 u'^2 - EA \omega^1 u'^1$$

$$u = N^i u^i \Rightarrow u' = N^i' u^{i''} \quad \omega = N^j \omega^j \Rightarrow \omega' = N^j' \omega^{j''}$$

$$\int_L EA N^j' \omega^j N^i' u^i dx = EA \omega^2 u'^2 - EA \omega^1 u'^1$$

From STRONG FORM TO ELEMENT STIFFNESS IN PHYSICAL SPACE now

$$EAu'' = 0 \quad \text{SUBJECT TO BCs}$$



$$\int_L EA \omega' u' dx = \int_L EA (\omega u')' dx = EA \omega u' \Big|_1^2 = EA \omega^2 u'^2 - EA \omega^1 u'^1$$

$$u = N^i u^i \Rightarrow u' = N^i' u^{i''} \quad \omega = N^j \omega^j \Rightarrow \omega' = N^j' \omega^{j''}$$

$$EAu' = EA\varepsilon = A \varepsilon \varepsilon = A \sigma = F$$

$$\int_L EA N^i' \omega^j N^i' u^i dx = EA \omega^2 u'^2 - EA \omega^1 u'^1 = \omega^2 F^2 + \omega^1 F^1$$

From STRONG FORM TO ELEMENT STIFFNESS \rightarrow IN PHYSICAL SPACE now

$$EAu'' = 0 \quad \text{SUBJECT TO BCs}$$



$$\int_L EA N^i \dot{w}^j N^j u^i dx = \omega^2 F^2 + w^i F^i$$

From STRONG FORM TO ELEMENT STIFFNESS \rightarrow IN PHYSICAL SPACE now

$$EAu'' = 0 \quad \text{SUBJECT TO BCs}$$



$$\int_L EA N^j \omega^j N^i u^i dx = \omega^2 F^2 + \omega^4 F^4$$

$$\omega^j \left[\int_L EA N^j N^i dx \right] u^i = \omega^2 F^2 + \omega^4 F^4$$

From STRONG FORM TO ELEMENT STIFFNESS IN PHYSICAL SPACE now

$$EAu'' = 0 \quad \text{SUBJECT TO BCs}$$



$$\int_L EA N^j \omega^j N^i u^i dx = \omega^2 F^2 + \omega^1 F^1$$

$$\omega^j \left[\int_L EA N^j N^i dx \right] u^i = \omega^2 F^2 + \omega^1 F^1 \quad \omega: \text{ARBITRARILY}$$

$$\text{e.g. } \omega^1 = 1, \omega^2 = 0$$

$$\text{e.g. } \omega^1 = 0, \omega^2 = 1$$

From STRONG FORM TO ELEMENT STIFFNESS \rightarrow IN PHYSICAL SPACE now

$$EAu'' = 0 \quad \text{SUBJECT TO BCs}$$



$$\int_L EA N^j \omega^j N^i u^i dx = \omega^2 F^2 + \omega^1 F^1$$

$$\omega^j \left[\int_L EA N^j N^i dx \right] u^i = \omega^2 F^2 + \omega^1 F^1 \quad \text{if } \omega: \text{ARBITRARILY}$$

$$\left[\int_L EA N^i N^j dx \right] u^j = F^1 \quad \Leftarrow \quad \begin{array}{l} \text{e.g. } \omega^1=1, \omega^2=0 \\ \text{e.g. } \omega^1=0, \omega^2=1 \end{array}$$

From STRONG FORM TO ELEMENT STIFFNESS \rightarrow IN PHYSICAL SPACE now

$$EAu'' = 0 \quad \text{SUBJECT TO BCs}$$



$$\int_L EA N^j \omega^j N^i u^i dx = \omega^2 F^2 + \omega^1 F^1$$

$$\omega^j \left[\int_L EA N^j N^i dx \right] u^i = \omega^2 F^2 + \omega^1 F^1 \quad \omega: \text{ARBITRARILY}$$

$$\left[\int_L EA N^1' N^1 dx \right] u^i = F^1 \quad \Leftarrow \quad \text{e.g. } \omega^1 = 1, \omega^2 = 0$$

$$\left[\int_L EA N^2' N^2 dx \right] u^i = F^2 \quad \Leftarrow \quad \text{e.g. } \omega^1 = 0, \omega^2 = 1$$

From STRONG FORM TO ELEMENT STIFFNESS  IN PHYSICAL SPACE now

$$EA u'' = 0 \quad \text{SUBJECT TO BCs}$$



$$\left[\int_L EA N^1' N^i dx \right] u^i = F^1$$

$$\left[\int_L EA N^2' N^i dx \right] u^i = F^2$$

From STRONG FORM TO ELEMENT STIFFNESS IN PHYSICAL SPACE now

$$EA u'' = 0 \quad \text{SUBJECT TO BCs}$$



$$\left[\int_L EA N^1' N^i dx \right] u^i = F^1$$

$$\left[\int_L EA N^2' N^i dx \right] u^i = F^2$$

$$\int_L EA N^1' N^1' dx u^1 + \int_L EA N^2' N^2' dx u^2 = F^1$$

From STRONG FORM TO ELEMENT STIFFNESS IN PHYSICAL SPACE now

$$EA u'' = 0 \quad \text{SUBJECT TO BCs}$$



$$\left[\int_L EA N^1' N^i dx \right] u^i = F^1$$

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From STRONG FORM TO ELEMENT STIFFNESS IN PHYSICAL SPACE now

$$EAu'' = 0 \quad \text{SUBJECT TO BCs}$$



$$\int_L EA N^1' N^1' dx u^1 + \int_L EA N^1' N^2' dx u^2 = F^1$$

$$\int_L EA N^2' N^1' dx u^1 + \int_L EA N^2' N^2' dx u^2 = F^2$$

$$EA \begin{bmatrix} \int_L N^1' N^1' dx & \int_L N^1' N^2' dx \\ \int_L N^2' N^1' dx & \int_L N^2' N^2' dx \end{bmatrix} \begin{bmatrix} u^1 \\ u^2 \end{bmatrix} = \begin{bmatrix} F^1 \\ F^2 \end{bmatrix}$$

From STRONG FORM TO ELEMENT STIFFNESS \rightarrow IN PHYSICAL SPACE now

$$EA u'' = 0 \quad \text{SUBJECT TO BCs}$$



$$EA \begin{bmatrix} \int_L N_1' N_1' dx & \int_L N_1' N_2' dx \\ \int_L N_2' N_1' dx & \int_L N_2' N_2' dx \end{bmatrix} \begin{bmatrix} u_1' \\ u_2' \end{bmatrix} = \begin{bmatrix} F_1' \\ F_2' \end{bmatrix}$$

From STRONG FORM TO ELEMENT STIFFNESS \rightarrow IN PHYSICAL SPACE now

$$EA u'' = 0 \quad \text{SUBJECT TO BCs}$$



$$EA \begin{bmatrix} \int_L N_1' N_1' dx & \int_L N_1' N_2' dx \\ \int_L N_2' N_1' dx & \int_L N_2' N_2' dx \end{bmatrix} \begin{bmatrix} u_1' \\ u_2' \end{bmatrix} = \begin{bmatrix} F_1' \\ F_2' \end{bmatrix}$$

NEXT, WE WRITE $x = x(\xi)$ USING SHAPE FUNCTIONS.

(ISOPARAMETRIC CONCEPT)

From STRONG FORM TO ELEMENT STIFFNESS \rightarrow IN PHYSICAL SPACE now

$$EA u'' = 0 \quad \text{SUBJECT TO BCs}$$



$$EA \begin{bmatrix} \int_L N_1' N_1' dx & \int_L N_1' N_2' dx \\ \int_L N_2' N_1' dx & \int_L N_2' N_2' dx \end{bmatrix} \begin{bmatrix} u_1' \\ u_2' \end{bmatrix} = \begin{bmatrix} F_1' \\ F_2' \end{bmatrix}$$

→ NEXT, WE WRITE $x = x(\xi)$ USING SHAPE FUNCTIONS.

→ ALTERNATIVELY, WE COULD HAVE WRITTEN THE SHAPE FUNCTIONS IN NATURAL SPACE ξ FROM SCRATCH!

(ISOPARAMETRIC)
CONCEPT

From STRONG FORM TO ELEMENT STIFFNESS \rightarrow IN PHYSICAL SPACE now

$$EA u'' = 0 \quad \text{SUBJECT TO BCs}$$



$$EA \begin{bmatrix} \int_L N^1' N^1' dx & \int_L N^1' N^2' dx \\ \int_L N^2' N^1' dx & \int_L N^2' N^2' dx \end{bmatrix} \begin{bmatrix} u^1 \\ u^2 \end{bmatrix} = \begin{bmatrix} F^1 \\ F^2 \end{bmatrix}$$

$$\begin{bmatrix} K^{11} & K^{12} \\ K^{21} & K^{22} \end{bmatrix} \begin{bmatrix} u^1 \\ u^2 \end{bmatrix} = \begin{bmatrix} F^1 \\ F^2 \end{bmatrix} \quad \text{and} \quad K^{ij} = EA \int_L N^i' N^j' dx$$

$$K^{ij} = EA \int_L n^i' n^j' dx$$

$$K^{ij} = EA \int_L n^i' n^j' dx = EA \int_L \frac{\partial n^i}{\partial x} \frac{\partial n^j}{\partial x} dx$$

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$$x = x(\xi) \Rightarrow dx = \frac{\partial x}{\partial \xi} d\xi \Rightarrow dx = J d\xi \text{ with } J = \frac{\partial x}{\partial \xi}$$

$$K^{ij} = EA \int_L n^i' n^j' dx = EA \int_L \frac{\partial N^i}{\partial x} \frac{\partial N^j}{\partial x} J d\xi$$

$$x = x(\xi) \Rightarrow dx = \frac{\partial x}{\partial \xi} d\xi \Rightarrow dx = J d\xi \text{ with } J = \frac{\partial x}{\partial \xi}$$

$$= EA \int_{-1}^1 \frac{\partial N^i}{\partial x} \frac{\partial N^j}{\partial x} J d\xi$$

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$$\frac{\partial N^i}{\partial x} = \frac{\partial N^i}{\partial \xi} \frac{\partial \xi}{\partial x} = \frac{\partial N^i}{\partial \xi} J^{-1}$$

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$$= EA \int_{-1}^1 \frac{\partial N^i}{\partial \xi} \frac{\partial N^j}{\partial \xi} J^{-1} J^{-1} J d\xi$$

$$\frac{\partial N^j}{\partial x} = \frac{\partial N^j}{\partial \xi} \frac{\partial \xi}{\partial x} = \frac{\partial N^j}{\partial \xi} J^{-1}$$

$$K^{ij} = EA \int_L n^i' n^j' dx = EA \int_L \frac{\partial N^i}{\partial x} \frac{\partial N^j}{\partial x} dx \quad \text{Jd\xi}$$

$$x = x(\xi) \Rightarrow dx = \frac{\partial x}{\partial \xi} d\xi \Rightarrow dx = J d\xi \text{ with } J = \frac{\partial x}{\partial \xi}$$

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NUMERICAL
INTEGRATION

using
GAUSS QUADRATURE

$$\int_{-1}^1 g(\xi) d\xi$$

$\overbrace{\hspace{100px}}$

$g(\xi)$

$$K^{ij} = EA \int_L n^i' n^j' dx \quad \xrightarrow{\text{PHYSICAL}} \text{RECALL:}$$

$$= EA \int_{-1}^1 \frac{\partial N^i}{\partial \xi} \frac{\partial N^j}{\partial \xi} \bar{J}^{-1} d\xi \quad \xrightarrow{\text{NATURAL}}$$

$$\int_{-1}^1 g(\xi) d\xi = \sum_{GP=1}^{GPE} g(\xi) \alpha_{GP}$$

\leftarrow Loop over GP

$$= EA \sum_{GP=1}^{GPE} \left\{ \left[\frac{\partial N^i}{\partial \xi} \quad \frac{\partial N^j}{\partial \xi} \quad \bar{J}^{-1} \right] \Big|_{GP} \times \alpha_{GP} \right\} \quad \vdots \quad \text{END}$$

)
eg.

WHAT YOU
SEE IN THE
CODE !

{ For $GP=1: GPE$
in
MATLAB
End

FINITE ELEMENT METHOD IN A NUTSHELL

Diff. Eq. \rightarrow 2^{ND.} O.D.E.

STRONG FORM

$$\int_0^L (EAu')' + b = 0$$

another source of approximation \rightarrow NUMERICAL INTEGRATION

ELEMENT-WISE QUANTITIES

PIECEWISE INTEGRALS (Solutions)

\rightarrow (I) Multiply By w \rightarrow (II) INTEGRATE

test function

Approximate Discretized Weak Form

APPROXIMATE FORM

WEAK FORM

INTEGRAL FORM

$$\int_0^1 w'u' dx = \int_0^1 w da$$

$$+ w(1)u'(1)$$

$$- w(0)u'(0)$$

PIECEWISE

DISCRETIZED FORM

Approximation

PostProcess

SOLVE

From Global To Elements

From INTEGRAL OVER THE DOMAIN

To SUBINTEGRALS

$$\int_0^1 \dots dx = \int_a^b \dots dx + \dots$$

$$[K][w] = [F]$$

ASSEMBLY

1D Finite Element Library

one-dimensional finite elements library

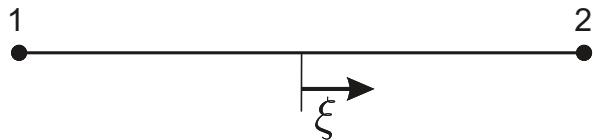


- one-dimensional 2-noded element (D1CU2N)
- one-dimensional 3-noded element (D1CU3N)
- one-dimensional 4-noded element (D1CU4N)
- one-dimensional 5-noded element (D1CU5N)
- one-dimensional 6-noded element (D1CU6N)
- one-dimensional 7-noded element (D1CU7N)
- one-dimensional quadrature rule
- one-dimensional constant body force nodal distribution

1D Finite Element Library

one-dimensional 2-noded element (D1CU2N)

Nodes



Node Number	Coordinate
	ξ
1	-1
2	1

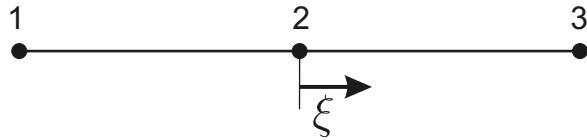
$$N^1 = -\frac{1}{2}(\xi - 1) \quad , \quad N^2 = \frac{1}{2}(\xi + 1)$$

$$N_{,\xi}^1 = -\frac{1}{2} \quad , \quad N_{,\xi}^2 = \frac{1}{2}$$

1D Finite Element Library

one-dimensional 3-noded element (D1CU3N)

Nodes



Node Number	Coordinate
	ξ
1	-1
2	0
3	1

$$N^1 = \frac{1}{2}\xi(\xi - 1) \quad , \quad N^2 = -(\xi - 1)(\xi + 1) \quad , \quad N^3 = \frac{1}{2}\xi(\xi + 1)$$

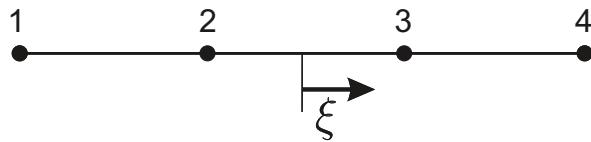
$$N_{,\xi}^1 = \frac{1}{2}(2\xi - 1) \quad , \quad N_{,\xi}^2 = -2\xi \quad , \quad N_{,\xi}^3 = \frac{1}{2}(2\xi + 1)$$

1D Finite Element Library

one-dimensional 4-noded element (D1CU4N)



Nodes



Node Number	Coordinate
	ξ
1	-1
2	-1/3
3	1/3
4	1

$$N^1 = -\frac{9}{16} (\xi + 1/3) (\xi - 1/3) (\xi - 1)$$

$$N^2 = \frac{27}{16} (\xi + 1) (\xi - 1/3) (\xi - 1)$$

$$N^3 = -\frac{27}{16} (\xi + 1) (\xi + 1/3) (\xi - 1)$$

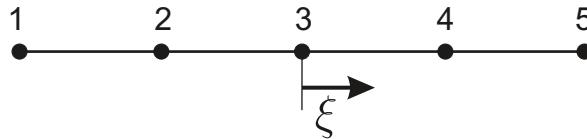
$$N^4 = \frac{9}{16} (\xi + 1) (\xi + 1/3) (\xi - 1/3)$$

1D Finite Element Library

one-dimensional 5-noded element (D1CU5N)



Nodes



Node Number	Coordinate
	ξ
1	-1
2	-1/2
3	0
4	1/2
5	1

$$N^1 = \frac{2}{3} (\xi + 1/2) \xi (\xi - 1/2) (\xi - 1)$$

$$N^2 = -\frac{8}{3} (\xi + 1) \xi (\xi - 1/2) (\xi - 1)$$

$$N^3 = 4 (\xi + 1) (\xi + 1/2) (\xi - 1/2) (\xi - 1)$$

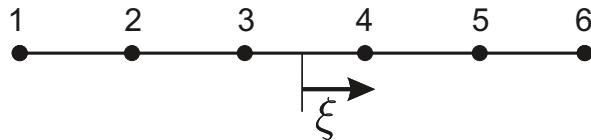
$$N^4 = -\frac{8}{3} (\xi + 1) (\xi + 1/2) \xi (\xi - 1)$$

$$N^5 = \frac{2}{3} (\xi + 1) (\xi + 1/2) \xi (\xi - 1/2)$$

1D Finite Element Library

one-dimensional 6-noded element (D1CU6N)

Nodes



Node Number	Coordinate
	ξ
1	-1
2	-3/5
3	-1/5
4	1/5
5	3/5
6	1

$$N^1 = -\frac{625}{768} (\xi + 3/5) (\xi + 1/5) (\xi - 1/5) (\xi - 3/5) (\xi - 1)$$

$$N^2 = \frac{3125}{768} (\xi + 1) (\xi + 1/5) (\xi - 1/5) (\xi - 3/5) (\xi - 1)$$

$$N^3 = -\frac{3125}{384} (\xi + 1) (\xi + 3/5) (\xi - 1/5) (\xi - 3/5) (\xi - 1)$$

$$N^4 = \frac{3125}{384} (\xi + 1) (\xi + 3/5) (\xi + 1/5) (\xi - 3/5) (\xi - 1)$$

$$N^5 = -\frac{3125}{768} (\xi + 1) (\xi + 3/5) (\xi + 1/5) (\xi - 1/5) (\xi - 1)$$

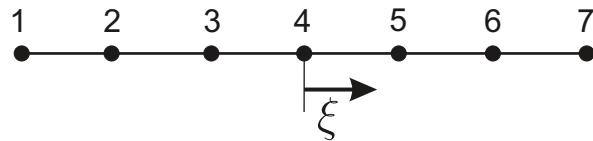
$$N^6 = \frac{625}{768} (\xi + 1) (\xi + 3/5) (\xi + 1/5) (\xi - 1/5) (\xi - 3/5)$$

1D Finite Element Library

one-dimensional 7-noded element (D1CU7N)



Nodes



Node Number	Coordinate
	ξ
1	-1
2	-2/3
3	-1/3
4	0
5	1/3
6	2/3
7	1

$$N^1 = \frac{81}{80} (\xi + 2/3) (\xi + 1/3) \xi (\xi - 1/3) (\xi - 2/3) (\xi - 1)$$

$$N^2 = -\frac{243}{40} (\xi + 1) (\xi + 1/3) \xi (\xi - 1/3) (\xi - 2/3) (\xi - 1)$$

$$N^3 = \frac{243}{16} (\xi + 1) (\xi + 2/3) \xi (\xi - 1/3) (\xi - 2/3) (\xi - 1)$$

$$N^4 = -\frac{81}{4} (\xi + 1) (\xi + 2/3) (\xi + 1/3) (\xi - 1/3) (\xi - 2/3) (\xi - 1)$$

$$N^5 = \frac{243}{16} (\xi + 1) (\xi + 2/3) (\xi + 1/3) \xi (\xi - 2/3) (\xi - 1)$$

$$N^6 = -\frac{243}{40} (\xi + 1) (\xi + 2/3) (\xi + 1/3) \xi (\xi - 1/3) (\xi - 1)$$

$$N^7 = \frac{81}{80} (\xi + 1) (\xi + 2/3) (\xi + 1/3) \xi (\xi - 1/3) (\xi - 2/3)$$

1D Finite Element Library

one-dimensional quadrature rule (i)

$$\int_{-1}^1 \{\bullet\} \, d\xi \approx \sum_{i=1}^{N_{\text{Gauss Points}}} \alpha_i \, \{\bullet\}|_{\text{Gauss Point}^i}$$

Gauss Point Number	Coordinate	Weight Factor
	ξ	α
1	0	2

Gauss Point Number	Coordinate	Weight Factor
	ξ	α
1	$-1/\sqrt{3}$	1
2	$1/\sqrt{3}$	1

Gauss Point Number	Coordinate	Weight Factor
	ξ	α
1	$-\sqrt{0.6}$	$5/9$
2	0	$8/9$
3	$\sqrt{0.6}$	$5/9$

1D Finite Element Library

one-dimensional quadrature rule (ii)

Gauss Point Number	Coordinate	Weight Factor
	ξ	α
1	-0.861 136 311 594 953	0.347 854 845 137 454
2	-0.339 981 043 584 856	0.652 145 154 862 546
3	0.339 981 043 584 856	0.652 145 154 862 546
4	0.861 136 311 594 953	0.347 854 845 137 454

Gauss Point Number	Coordinate	Weight Factor
	ξ	α
1	-0.906 179 845 938 664	0.236 926 885 056 189
2	-0.538 469 310 105 683	0.478 628 670 499 366
3	0.000 000 000 000 000	0.568 888 888 888 889
4	0.538 469 310 105 683	0.478 628 670 499 366
5	0.906 179 845 938 664	0.236 926 885 056 189

Gauss Point Number	Coordinate	Weight Factor
	ξ	α
1	-0.932 469 514 203 152	0.171 324 492 379 170
2	-0.661 209 386 466 265	0.360 761 573 048 139
3	-0.238 619 186 083 197	0.467 913 934 572 691
4	0.238 619 186 083 197	0.467 913 934 572 691
5	0.661 209 386 466 265	0.360 761 573 048 139
6	0.932 469 514 203 152	0.171 324 492 379 170

1D Finite Element Library

one-dimensional quadrature rule (iii)

Gauss Point Number	Coordinate	Weight Factor
	ξ	α
1	-0.949 107 912 342 759	0.129 484 966 168 870
2	-0.741 531 185 599 394	0.279 705 391 489 277
3	-0.405 845 151 377 397	0.381 830 050 505 119
4	0.000 000 000 000 000	0.417 959 183 673 469
5	0.405 845 151 377 397	0.381 830 050 505 119
6	0.741 531 185 599 394	0.279 705 391 489 277
7	0.949 107 912 342 759	0.129 484 966 168 870

1D Finite Element Library

one-dimensional constant body force nodal distribution

$$f^i = \frac{\int_{-1}^1 N^i d\xi}{\int_{-1}^1 d\xi}$$

Node number	1	2
Weight factor	1/2	1/2

Node number	1	2	3
Weight factor	1/6	4/6	1/6

Node number	1	2	3	4
Weight factor	1/8	3/8	3/8	1/8

Node number	1	2	3	4	5
Weight factor	7/90	32/90	12/90	32/90	7/90

Node number	1	2	3	4	5	6
Weight factor	19/288	25/96	25/144	25/144	25/96	19/288

Node number	1	2	3	4	5	6	7
Weight factor	41/840	18/70	9/280	68/210	9/280	18/70	41/840